

Introduction and Objectives

Introduction

- Due to the inherent defects and brittleness of natural stone, brittle fracture would occur in stone beams without obviously visible signs. To improve the general behaviour and safety of the stone structure, bare stone flexural members need to be strengthened to improve their performance.
- The existing methods for strengthening stone beams have some limitation.

External steel wires → Damage the original appearance of the structure; poor durability.

External prestressing → Large site limitations; difficulties in operation.

NSM CFRP bars → Difficulties in overhead handling and site grooving.

Objectives

- Propose a strengthening technique for stone beams-external prefabricated prestressed CFRP-reinforced stone plates, and present an investigation on the performance of strengthened stone beams.
- Investigate the effects of CFRP reinforcement ratio and prestress level on the strengthening effect.

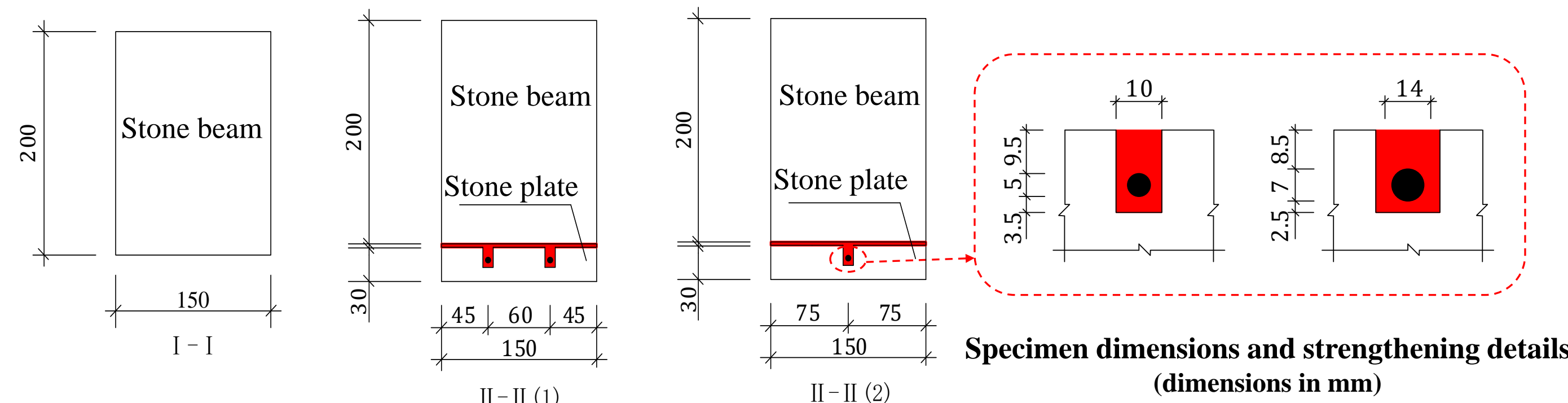
Methodology

Specimen information

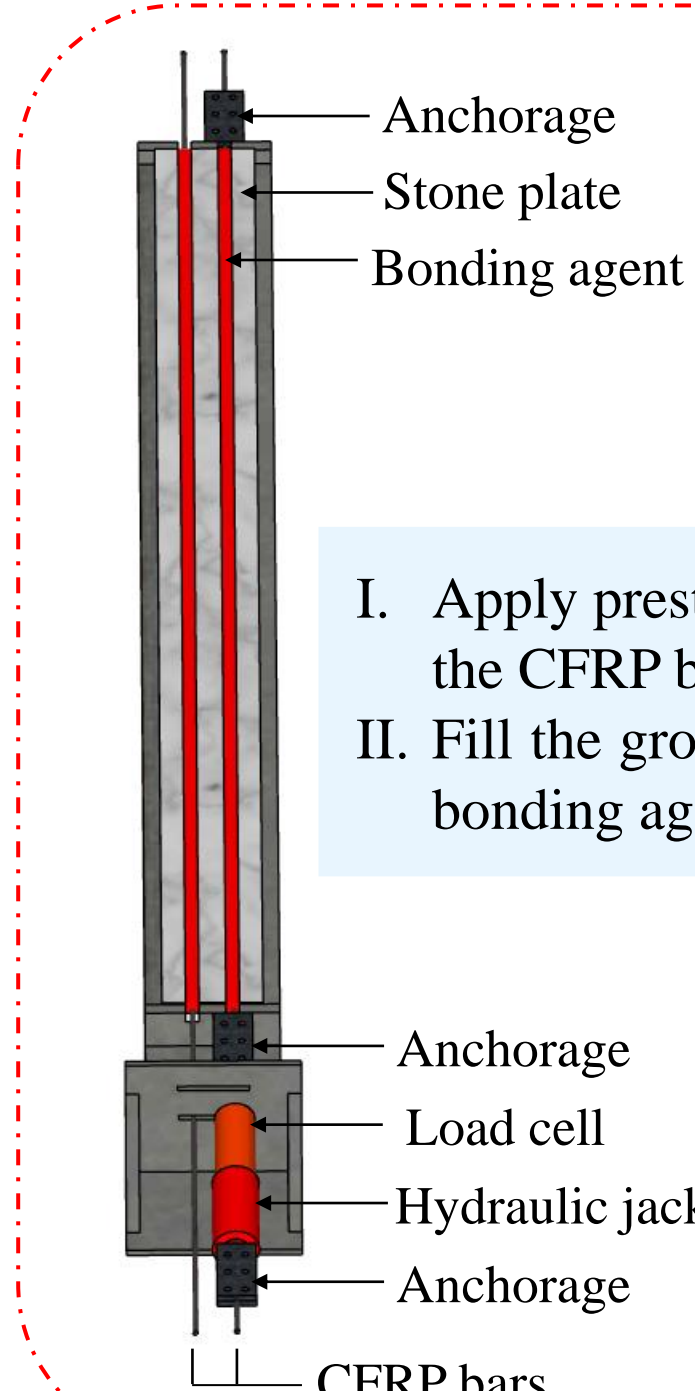
- Stone beam: 150 × 200 × 2000 (width(b) × height(h) × length (l)).
- Prefabricated prestressed CFRP-reinforced stone plate: 2000 × 150 × 30 (length (l) × width(b) × thickness (t)).

| Specimen | d_f | n | $\rho_f(\%)$ | δ_f |
|-----------|-------|-----|--------------|------------|
| P1 | — | — | — | — |
| P-1d5-0.2 | 5 | 1 | 0.45 | 0.20 |
| P-2d5-0.2 | 5 | 2 | 0.95 | 0.20 |
| P-1d7-0.2 | 7 | 1 | 0.91 | 0.20 |
| P-2d7-0.1 | 7 | 2 | 1.93 | 0.10 |
| P-2d7-0.2 | 7 | 2 | 1.93 | 0.20 |
| P-2d7-0.3 | 7 | 2 | 1.93 | 0.30 |

d_f : diameter of CFRP bar; n : number of CFRP bars; ρ_f : reinforcement ratio; δ_f : prestress level



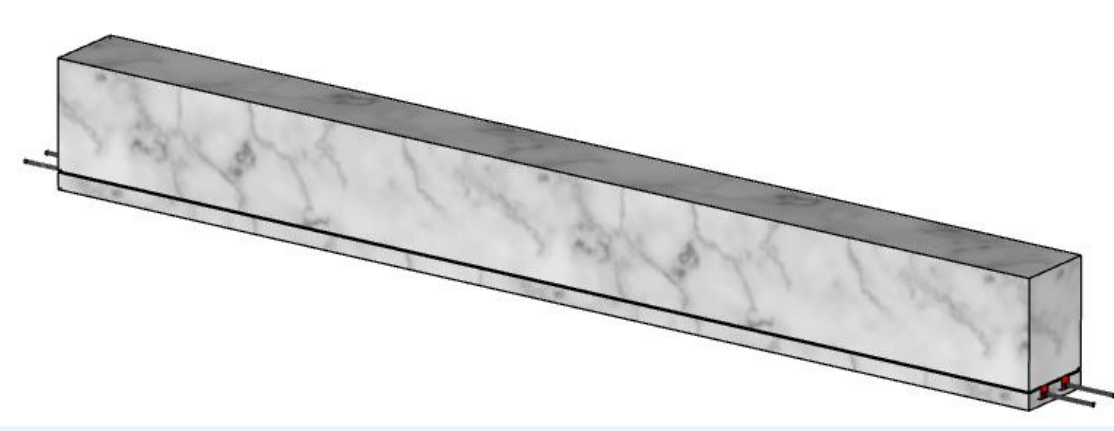
Specimen preparation



- Apply prestress to the CFRP bars.
- Fill the grooves with bonding agent.



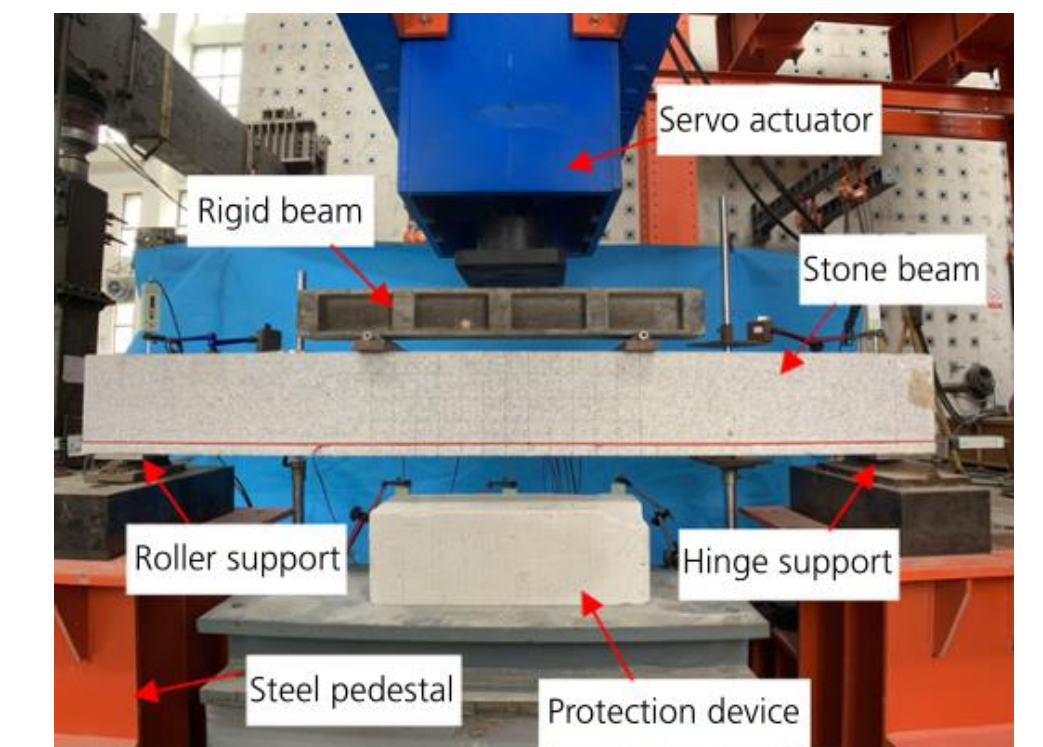
- Maintain prestress by the device.
- Bond the prefabricated stone plate to the stone beam with bonding agent.



- When the strain of CFRPs attained a stabilized value, the anchorage system was disassembled.
- The prestress of CFRP bars was effectively transferred to the strengthened stone beam.

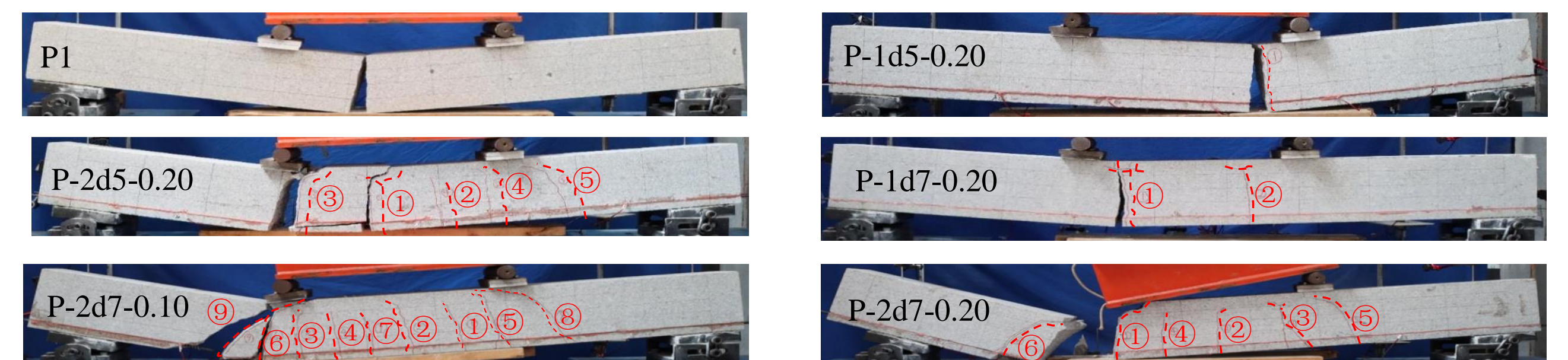
Testing Method

- Four-point bending tests were conducted under displacement control.
- The loading rate was set as 0.2 mm/min prior to cracking followed by a rate of 0.4 mm/min until the end of testing.



Experimental results and discussion

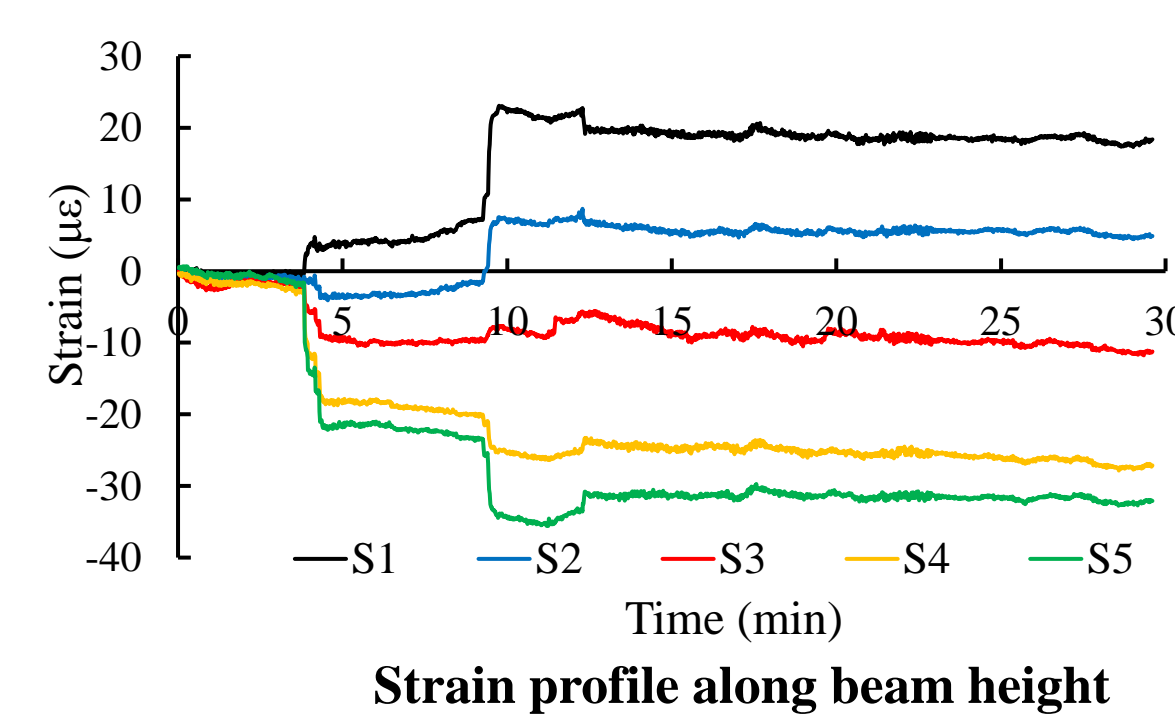
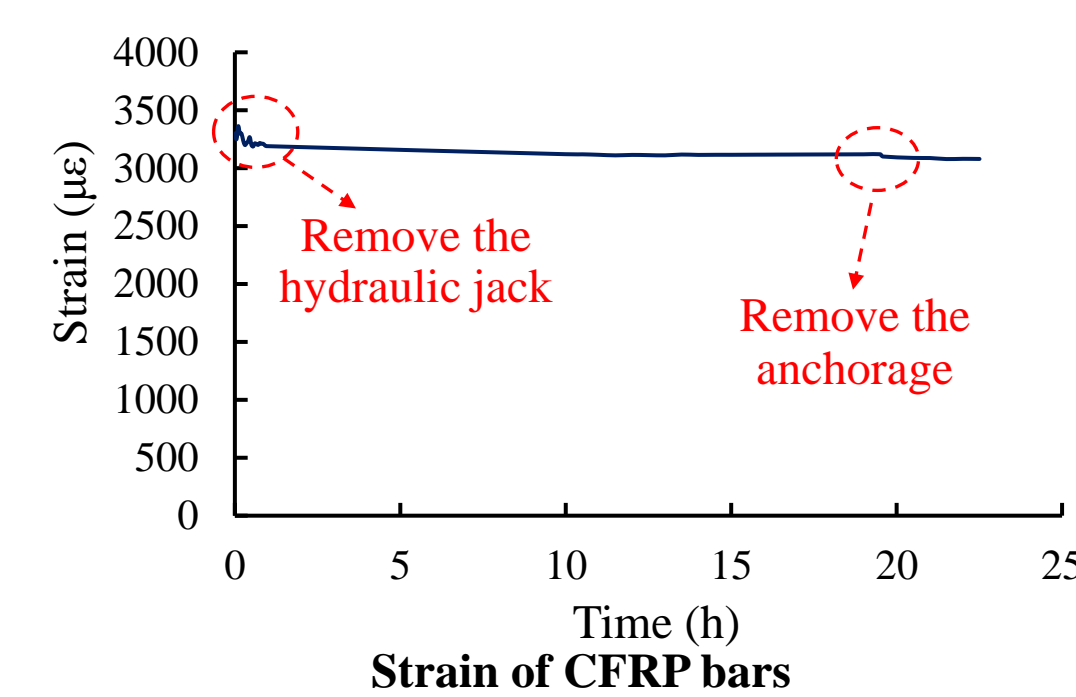
Failure mode



Note: The numbers on the specimens indicate the order of crack formation.

Strain variation in prestress release process

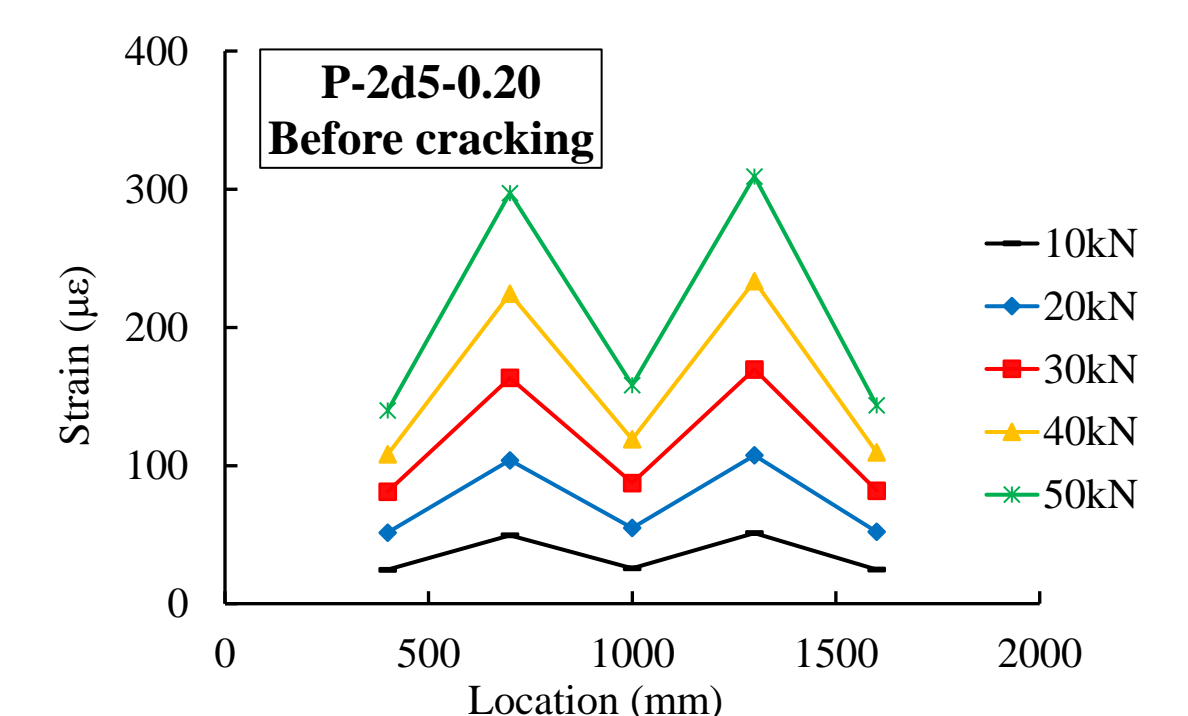
- The stone strain distribution at midspan was in line with the linear law, and the neutral axis was roughly half the height of the strengthened stone beams.



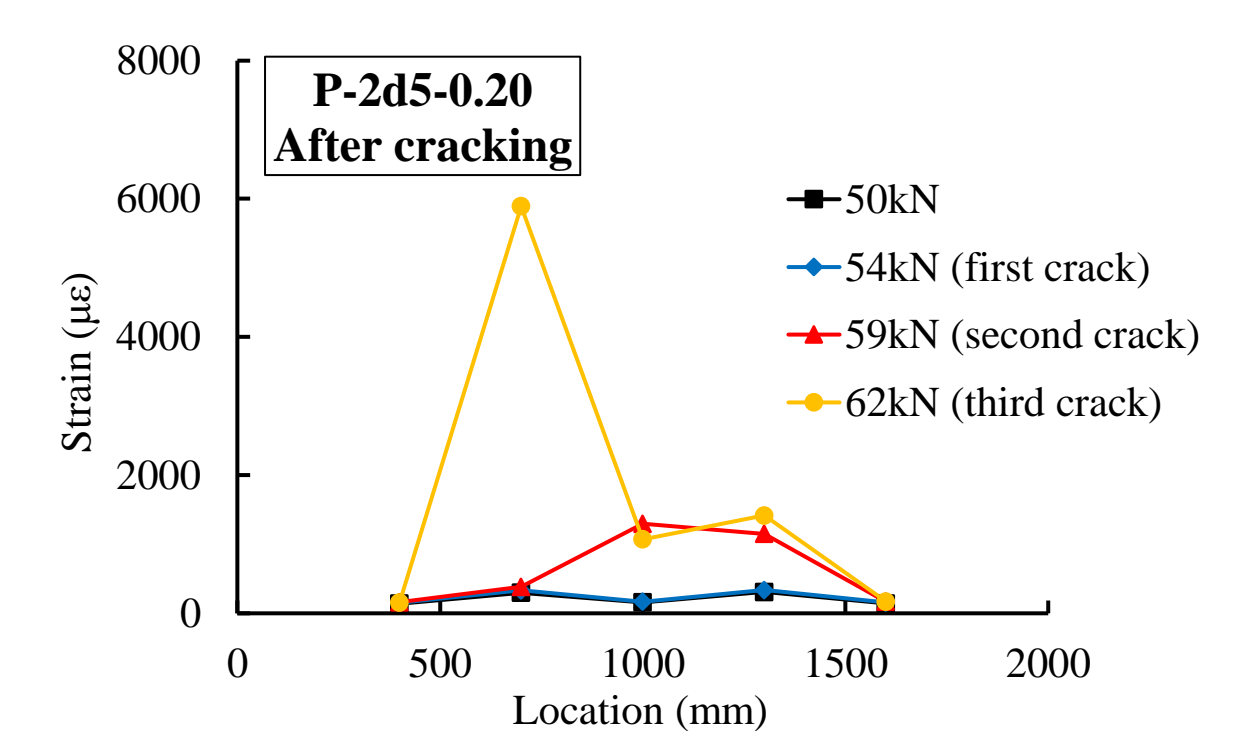
S1-S5: strain along the height direction of the stone beam span mid-section.

Strain in CFRP bars

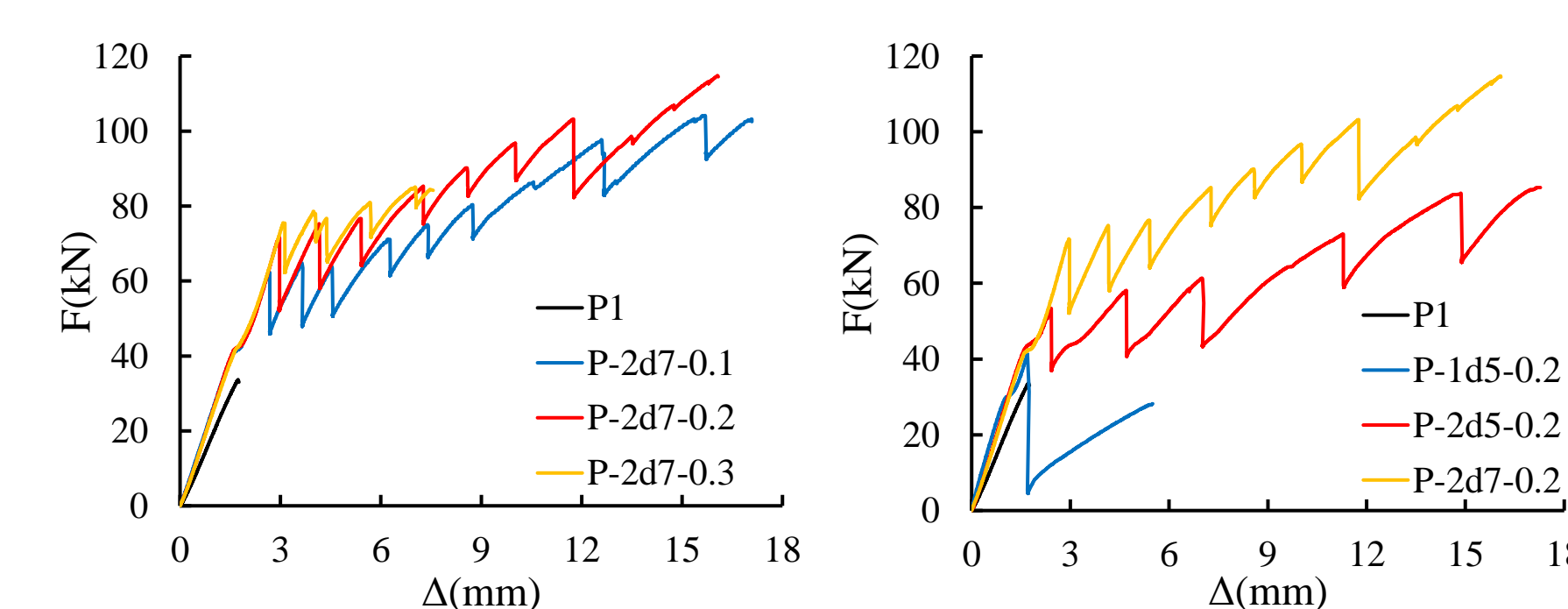
The deformation of CFRP bars and stone beam were coordinated.



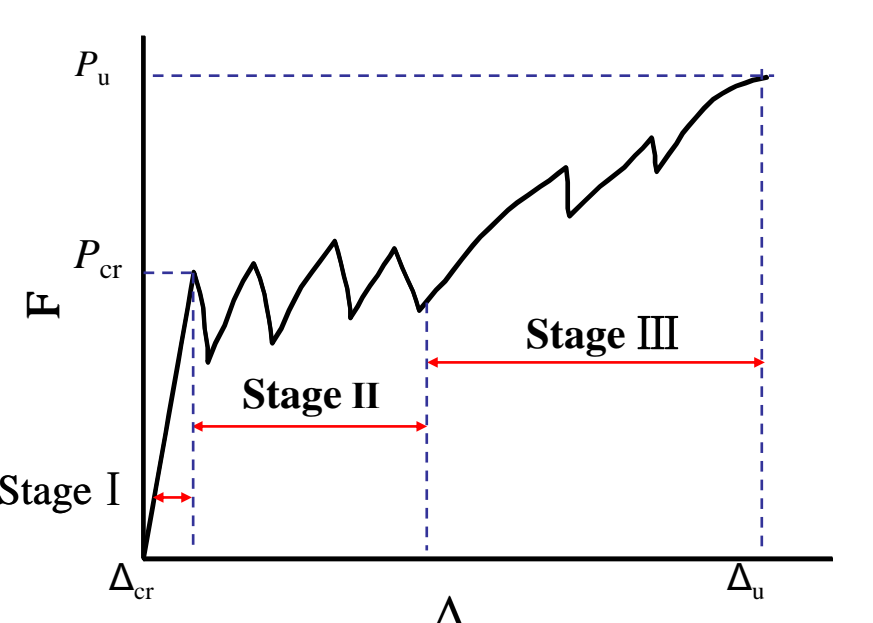
The CFRP bars took the strain instead of the stone beam.



Load-deflection response



Typical load-deflection responses of the strengthened stone beam



Conclusions

- The strengthening technique could effectively change the failure mode of bare stone beams from brittle fracture to a more ductile behavior.
- The strengthened stone beams showed significantly improved flexural strengths and deformation capacities. The maximum increase in the moment capacity was about 113.6% compared with that of the bare stone beam.
- An increase in the CFRP reinforcement ratio and prestress level significantly increased the moment capacity of the strengthened stone beams.